ASSESSMENT OF LIQUEFACTION POTENTIAL FOR SEISMIC RISK REDUCTION IN NORTH-EAST INDIA

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Abstract — The study of liquefaction potential of a region is of utmost importance regarding the safety of both life and property. Soil liquefaction occurs when there is loss of strength and stiffness in saturated and cohesion less soil due to increase in pore water pressure. The strength of the soil is sometimes reduced by earthquake shaking or rapid loading. Liquefaction causes soil failures which leads to severe damages to structures supported on such grounds leading to significant economic losses. The main purpose of the present study is to analyse liquefaction of some selected sites of Northeast India which falls in the zone of highest seismic risk zone level (Zone V in India) using bore log data of 95 boreholes upto a depth of 15m. The liquefaction analysis is carried out with the help of 2 different methods and results are compared. Factor of safety versus depth curves plotted for showing the change in soil liquefaction with increasing depth. The methods used for analysis are simplified approach by Seed and Idriss (1971) and IS code procedure for evaluation of liquefaction potential (2016). More detailed study can be done in future and also various mitigation strategies can be put forward to reduce the impact of hazard.

Keywords— Borehole data, Cyclic resistance ratio, Cyclic stress ratio, Factor of safety, Liquefaction.

I. INTRODUCTION

Liquefaction is typically associated with earthquake-induced shaking that causes the ground to lose its bearing strength and act like a fluid. This can cause entire buildings to topple and cars to get sucked in. It can cause surface layers to slide downhill, damaging roads and rupturing distributed infrastructure systems like water and gas lines. Earthquakes seismic event caused number of disturbances in the ground which could harm or damage structural stability leading to liquefaction. The construction of building near water bodies use retaining walls which are heavily dependent on the strength and stiffness of the soil. Once the soil gets liquefied, the retaining wall collapse which could cause landslide. The best known cases of foundation failures due to liquefaction are those that occurred during the 1964 earthquake in Niigata, Japan (Kishida, 1966).

Liquefaction is typically characterized by generation of excess pore pressure under undrained loading. The tendency of loose sands to densify under drained loading is well known. When loose sands are saturated and loaded under undrained conditions, the tendency to densify causes an increase in pore pressure, leading to a decrease in effective confining pressure. This lowers the shear strength of the soil, causing it to liquefy.

Guwahati city lies on a very high seismic zone (zone V) as per IS: 1893 along with the entire North Eastern region vulnerable to major earthquakes. The drifting of the Indian subcontinent also plate towards the Eurasian plate with the passage of time has been one of the sole reasons of earthquake occurrence in this part of the world. The northward moving Indian plate at the rate of 20mm/year can provide earthquakes of magnitudes 8 and above every few hundred years. Most part of the city is built on soft sediments it can reasonably amplify the earthquake ground motion in case of a seismic event. Such areas have high risk of liquefaction occurrence of a strong intensity earthquake.
II. BACKGROUND INFORMATION

The devastating effect of liquefaction was first observed during various worldwide earthquakes including the 1987 great Assam Earthquake and the 1950 Assam earthquake both of magnitude 8.7 approximately. These instances of liquefaction necessitate the need to evaluate liquefaction susceptibility of an area. Evaluations of soil liquefaction potential using laboratory as well as several field techniques like standard penetration test (SPT), cone penetration test(CPT), shear wave velocity(Vs) etc. have become popular among the practising engineers.

Analysis of Guwahati city was performed by Nath et al. (2008) and PGA falls in the range 0.22g-1.27g with amplification factor 2-10. Different research developments in determination of liquefaction potential using the deterministic as well as the probabilistic approach have evolved due to the continued effort of many research workers. The developments started with the pioneering work of Seed and Idriss in 1971, and gradually by Seed and Idriss (1982) and Seed et al. (1985), by Liao and Whitman (1986), by Idriss (1999), NCEER Workshops- (1996 and 1997), by Toprak et al. (1999), by Juang et al. (2002), by Cetin et al. (2004), by Idriss and Boulanger (2004, 2006) etc.

A detailed study on the status of liquefaction potential of Guwahati city was first attempted by Raghukanth and Dash (2010) based on the scenario earthquakes of this region. According to their research findings, the central part of the city near Dispur, Chandmari and along the Guwahati-Shillong (GS) road is highly vulnerable to liquefaction. The FOS was found to be less than 1 in almost all the places under study which means almost entire Guwahati city is susceptible to liquefaction.

III. STUDY AREA

Guwahati city which lies in the North Eastern region of India, falls in the highest seismic risk zonal level i.e. zone V in India. Liquefaction of soil and its associated damages have been widely observed in many previous earthquakes in North Eastern region of Assam. The entire North Eastern region has witnessed many high magnitude earthquakes in the past decades.

Guwahati, is India’s biggest city that falls in zone V. Guwahati is also amongst the list of cities, Government of India has shortlisted to be developed as “smart city”. For this reason, the city attracts a lot of infrastructural growth in the times to come. Also recent developments have led to more construction of oil refineries, industries, hospitals, flyovers, multiplex halls etc. These heavy buildings require pile foundations which are embedded deep into the ground because of soil support. But if the soil is not strong enough to support then the foundation buckle which leads to collapsing of the structure. A major earthquake in this region shall lead to extensive damages to life and property.
Fig 4: Collected borelog of one borehole.

A. Procedure of Simplified Approach Method by Seed and Idriss (1971)

The procedure allows the factor of safety (FOS) against liquefaction to be calculated for soil structure at a given depth.

With the help of specific gravity and void ratio, the saturated unit weight and submerged unit weight was found out by:

\[ \gamma_{\text{sat}} = \left( g + \frac{c}{1 + e} \right) \gamma_w \]
\[ \gamma_{\text{sub}} = \gamma_{\text{sat}} - \gamma_w \]

Then total vertical stress (\(\sigma_v\)) and effective overburden pressure (\(\sigma'_v\)) was found out by:

\[ \sigma_v = \gamma_{\text{sat}} \times H_1 \]
\[ \sigma'_v = \gamma_{\text{sat}} \times H_2 \]

Then, value of stress reduction factor (\(r_d\)) was calculated for depth H from Seed and Idriss curve (1971) shown in fig 3.

Peak ground surface acceleration (\(a_{\text{max}}\)) is considered as 0.36g.

Now, cyclic stress ratio (CSR) is given by:

\[ \text{CSR} = 0.65 \times \frac{a_{\text{max}}}{g} \times \frac{\sigma_v}{\sigma'_v} \times r_d \]

Observed value of no. of blows (\(N_m\)) is taken from bore log chart.

Now, overburden correction factor (\(C_N\)) was calculated using the formula:

\[ C_N = 9.81 \times \frac{1}{\sqrt{\frac{\sigma'_v}{\sigma_v}}} \]

Actual hammer energy (\(E_m\)) was considered to be 72% of theoretical free fall energy (\(E_{\text{eff}}\)).

Now, corrected value of no. of blows (\(N_{160}\)) was found out and hammer efficiency of 60% was taken:

\[ N_{160} = N_m \times C_N \times \frac{E_m}{0.6 E_{\text{eff}}} \]

Then, the value of CRR was taken from \(N_{160}\) versus CRR curve shown in Fig 4.
Fig 6: - $(N_{1})_{60}$ vs. CRR curve

Finally, factor of safety was calculated by:

$$FOS = \frac{CRR}{CSR}$$

If FOS is less than 1, soil will undergo liquefaction.
If FOS is more than 1, soil will not undergo liquefaction.


The procedure allows the factor of safety (FOS) against liquefaction to be calculated for soil structure at a given depth.

With the help of specific gravity and void ratio, the saturated unit weight and submerged unit weight was found out by:

$$\gamma_{sat} = \frac{(g + e)}{1 + e} \gamma_w$$

$$\gamma_{sub} = \gamma_{sat} - \gamma_w$$

Then total vertical stress($\sigma_v$) and effective overburden pressure($\sigma'_v$) was found out by:

$$\sigma_{v1} = \gamma_{sat} \times H_1$$

$$\sigma'_{v1} = \gamma_{sat} \times H_1$$

Then, value of stress reduction factor ($r_d$) was calculated for depth H by:

$$1 - (0.00765 \times H)$$

Peak ground surface acceleration ($a_{max}$) is considered as 0.36g.

Now, cyclic stress ratio (CSR) is given by:

$$CSR = 0.65 \times \frac{a_{max}}{g} \times \frac{\sigma_v}{\sigma'_v} \times r_d$$

Observe value of no. of blows ($N_{60}$) is taken from bore log chart.
Now, the value of $(N_1)_{60}$ is calculated by:

$$(N_1)_{60} = C_N \times N_{60}$$

Then, the value of CRR was found out from the $(N_1)_{60}$ vs. CRR curve.
Finally, factor of safety was calculated by:

$$FOS = \frac{CRR}{CSR}$$

If FOS is less than 1, soil will undergo liquefaction.
If FOS is more than 1, soil will not undergo liquefaction.

Here, $\gamma_d$ = dry unit weight of soil in KN/m³,$\gamma_{sat}$ = saturated unit weight of soil in KN/m³, $\gamma_w$ = unit weight of water in KN/m³, $H$ = depth of bore hole in m, $\sigma_v$ = vertical stress in KN/m², $\sigma'_v$ = effective overburden pressure in KN/m², $a_{max}$ = peak ground surface acceleration, $g$ = acceleration due to gravity, $r_d$ = value of stress reduction factor, CSR = cyclic stress ratio, $C_N$ = observed values of number of blows, $Em$ = actual hammer energy, $F_{eff}$ = theoretical free fall energy, $(N_{1})_{60}$ = corrected values of number of blows normalised for 100kPa overburden pressure and hammer efficiency of 60%., CRR = cyclic resistance ratio, FOS = factor of safety.

FOS is calculated by using the same method as above for all the depths for borehole data collected which is represented in the table below.

The factor of safety calculations of 8 boreholes out of 95 are shown below.

<table>
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<tr>
<th>Depth</th>
<th>$\sigma_v$(kN/m²)</th>
<th>$\sigma'_v$(kN/m²)</th>
<th>$r_d$</th>
<th>CSR</th>
<th>$C_N$</th>
<th>$(N_1)_{60}$</th>
<th>FOS</th>
<th>Liquefaction</th>
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<td>28.74</td>
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Table 1: FOS by Seed and Idriss
### Table 2: FOS by IS Code Procedure

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<th>CSR</th>
<th>N_u</th>
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### Table 3: FOS by Seed and Idriss

### Table 4: FOS by IS Code Procedure

### Table 5: FOS by Seed and Idriss

### Table 6: FOS by IS Code Procedure
### D. BOREHOLE IV

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<th>No.</th>
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Table 7: FOS by Seed and Idriss

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<th>$\tau_b$</th>
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Table 8: FOS by IS Code Procedure

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Table 9: FOS by Seed and Idriss

Table 10: FOS by IS Code Procedure

Table 11: FOS by Seed and Idriss
Comparing the results of 8 boreholes obtained from both the methods.
A. Borehole I

B. Borehole II

C. Borehole III

D. Borehole IV

E. Borehole V

F. Borehole VI

G. Borehole VII

Fig 7: FOS vs Depth

Fig 8: FOS vs Depth

Fig 9: FOS vs Depth

Fig 10: FOS vs Depth

Fig 11: FOS vs Depth

Fig 12: FOS vs Depth

Depth that will undergo liquefaction

Depth that will not undergo liquefaction

DEPTH (m)

FOS

DEPTH (m)

FOS

DEPTH (m)

FOS

DEPTH (m)

FOS

DEPTH (m)

FOS
As per IS code 1893 (2002) Guwahati lies in Zone V. The peak ground acceleration for Guwahati is 0.36g for an 8.1 magnitude earthquake. The factor of safety against liquefaction is determined for all the 95 boreholes using Seed and Idriss and IS Code Procedure. Out of the 95 boreholes 59 boreholes was found to be susceptible to liquefaction by both the methods. The lowest factor of safety against liquefaction among the layers is considered to be the factor of safety for that borehole.

From the calculation it is observed that areas like Chandmari, zoo road, Sixmile, Beltola, Dispur, G.S. Road, Uzanbazar, jalukbari and Bharalamukh are most susceptible to liquefaction.

Although the factor of safety are calculated for all 95 boreholes but tables 1 to 16 shows factor of safety withdepth of only 8 boreholes for both the methods. A comparison of factor of safety between the two methods is shown in figs 7 to 14. It is observed that for all the 95 boreholes both the methods gives almost the same results. Hence the IS code procedure may be preferred as it is a much simpler method as compared to Seed & Idriss for evaluation of liquefaction potential.

The total thickness of soil up to 15m is susceptible to liquefaction, which means that Guwahati city area is most vulnerable to liquefaction related hazards during future earthquakes of magnitude more than 8. From the map it is observed that the areas areas like Chandmari, Zoo Road, Sixmile, Beltola, Dispur, G.S. Road, Uzanbazar, Jalukbari and Bharalamukh...
are most susceptible to liquefaction. Both the northern and southern bank of the city is susceptible to liquefaction. Also, a relative comparison of factor of safety using Seed & Idriss and IS Code Procedure method is also made. Thus it is hoped that this paper contributes in making a broader microzonation map of Guwahati.

VIII. REFERENCE


19. Sahib Ahmed et al.(2015), conducts the standard penetration test at Lucknow

20. Seed and Idriss(1971); Seed and Idriss(1982); and Seed et al.(1985); Liao and Whitman(1986), Idriss(1999); Toprak et al. (1999); Juang et al., (2002); Cetin et al. (2004), Idriss and Boulanger(2004, 2006), determination of liquefaction potential using the deterministic as well as probabilistic approach.

